

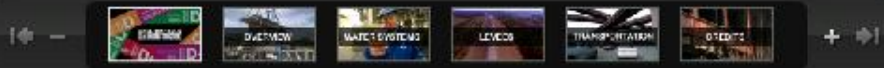
Geogrid Reinforcement in Flexible Pavements

Bill Tiltman

Tensar International Corporation

October 6, 2010



SEARCH


2009 Grades

Aviation	D
Bridges	C
Dams	D
Drinking Water	D-
Energy	D+
Hazardous Waste	D
Inland Waterways	D-
Levees	D-
Public Parks and Recreation	C-
Rail	C-
Roads	D-
Schools	D
Solid Waste	C+
Transit	D
Wastewater	D-

America's Infrastructure GPA: **D**
 Estimated 5 Year Investment
 Need: **\$2.2 Trillion**

The Infrastructure Roundtables

Read the Civil Engineering report on these events about the 5 Key Solutions

It's Your State

What's the state of your State's infrastructure? Find out now

Infrastructure in the News

Join Our Facebook Group:
 "Save America's Infrastructure"

ASCE Member? Be a Key Contact

Paying More for Gas is Good for Us

Meet the New Kid on the Blogroll

ASCE and Building America's Future Call

Take Our Poll

Five categories received D-
 Which is the most urgent?



TRANSPORTATION
ROADS | 2009
GRADE | **D-**

RAISING THE GRADES SOLUTIONS THAT WILL WORK NOW

A = Exceptional
B = Good
C = Mediocre
D = Poor
F = Failing

AMERICA'S INFRASTRUCTURE G.P.A. **D**

ESTIMATED 5-YEAR FUNDING REQUIREMENTS FOR BRIDGES AND ROADS

Total investment needs
\$990 BILLION

Estimated spending
\$380.5 BILLION
Projected shortfall
\$549.5 BILLION



- ★ **REFORM** the federal highway program to emphasize performance management, cost-benefit analysis, and accountability;
- ★ **DIRECT** federal transportation policies, programs, and resources to enhance U.S. global competitiveness, interstate commerce, passenger travel, and emergency preparedness;
- ★ **INCREASE** spending significantly at all levels of government to repair, improve, and expand the nation's surface transportation system;
- ★ **INCREASE** funding for long-term, advanced highway research;
- ★ **ADDRESS** the long-term viability of fuel taxes for transportation funding, and explore the viability of the most promising options to strengthen this funding;
- ★ **ESTABLISH** a national policy goal of achieving zero deaths on America's roadways and **INCREASE** funding in the Highway Safety Improvement Program by 10%.

- “33% of America's major roads are in poor or mediocre condition...”
- “The current spending level of \$70.3 billion for highway capital improvements is well below the estimated \$186 billion needed annually to substantially improve the nation's highways. ”

ESTIMATED 5-YEAR FUNDING REQUIREMENTS FOR BRIDGES AND ROADS

Total investment needs
\$990 BILLION

Estimated spending
\$380.5 BILLION
Projected shortfall
\$549.5 BILLION



Some Causes of Premature Road Failure

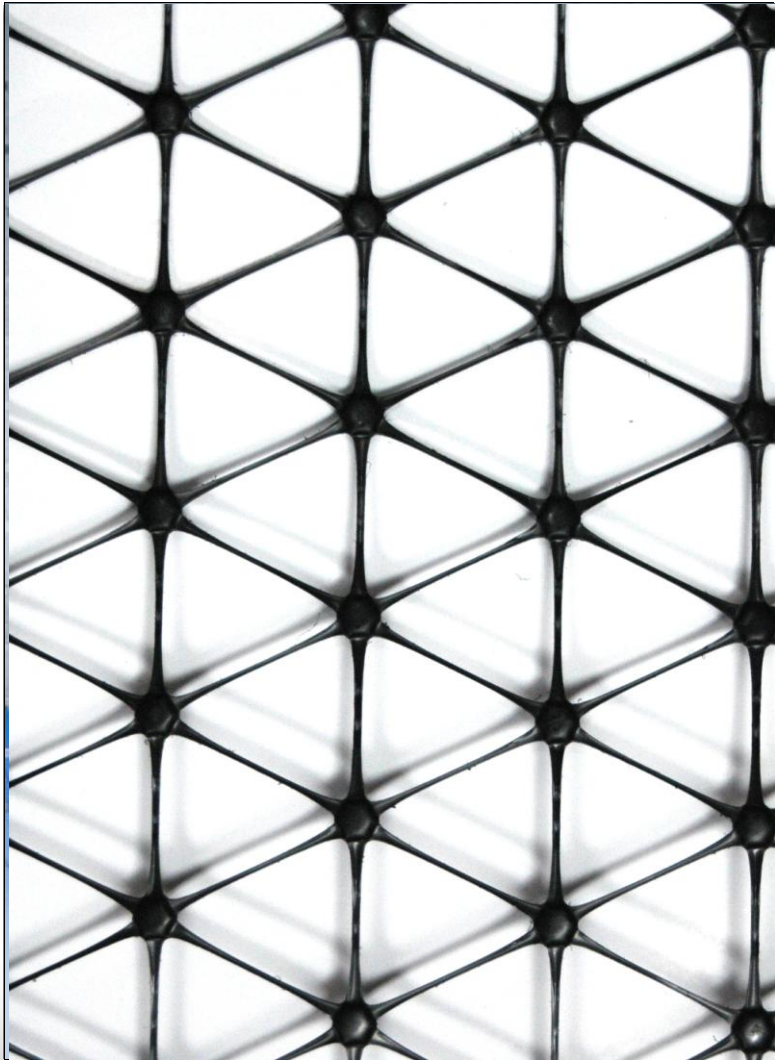
1. Weak Subgrade
2. Poor Drainage
3. Excessive Loading
4. Construction Errors
5. Material Failure
6. Unintended Use
7. Construction Damage to Subgrade
8. Regional Issues
 - Heaving, Thermal Cracking





Geogrid Introduction

Geogrid



- Biaxial Geogrids
 - Extruded
 - Woven
 - Welded
 - Geogrid Composites
- Triaxial Geogrid
 - Extruded

- **Subgrade Improvement**
 - Undercut reduction in soft soils
 - Improved construction platform for road construction
 - Protection of sensitive subgrade soils

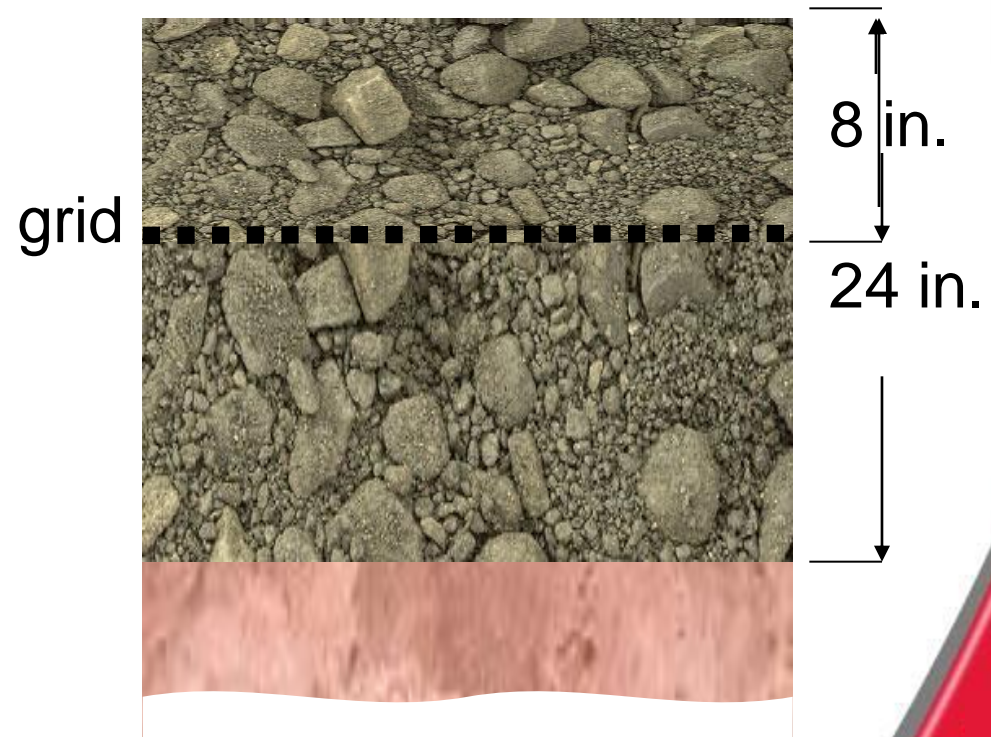
- **Pavement Base Reinforcement**
 - Stiffening of base aggregate layer
 - Reduction of pavement section
 - Extended life of pavement

SUBGRADE IMPROVEMENT

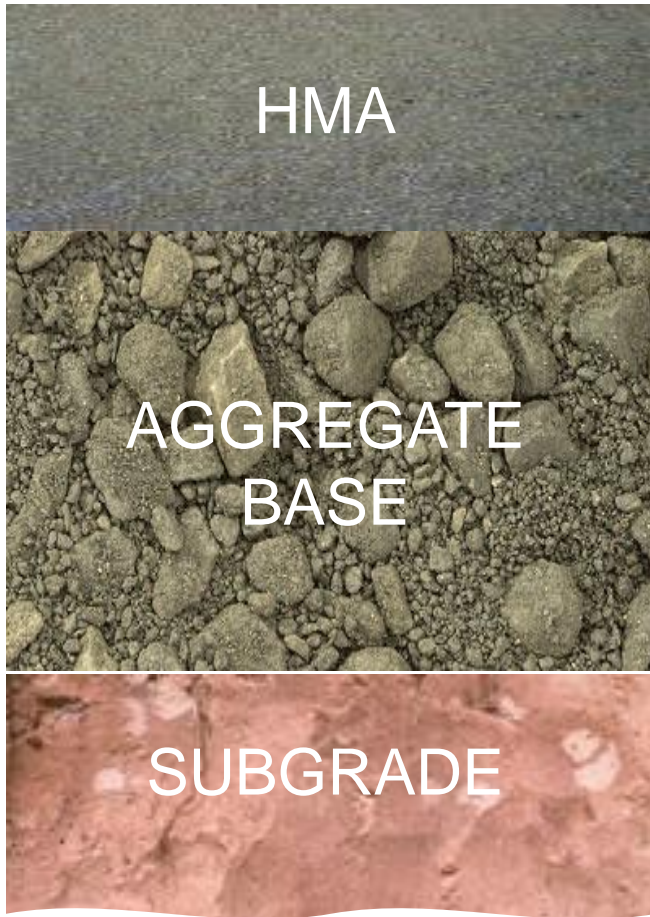


SOFT SUBGRADE

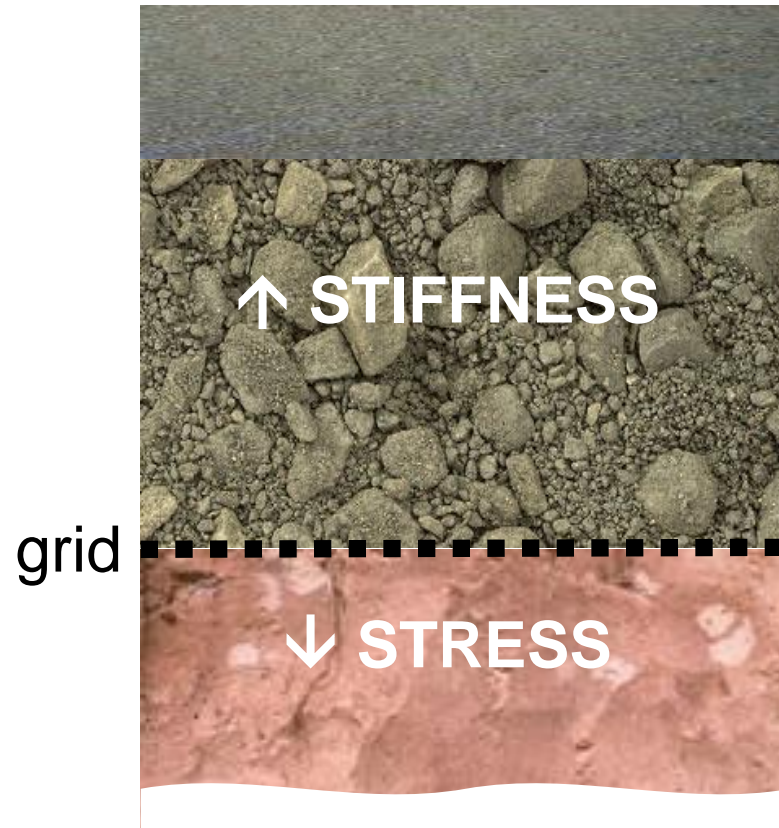
$M_r < 5,000$ psi



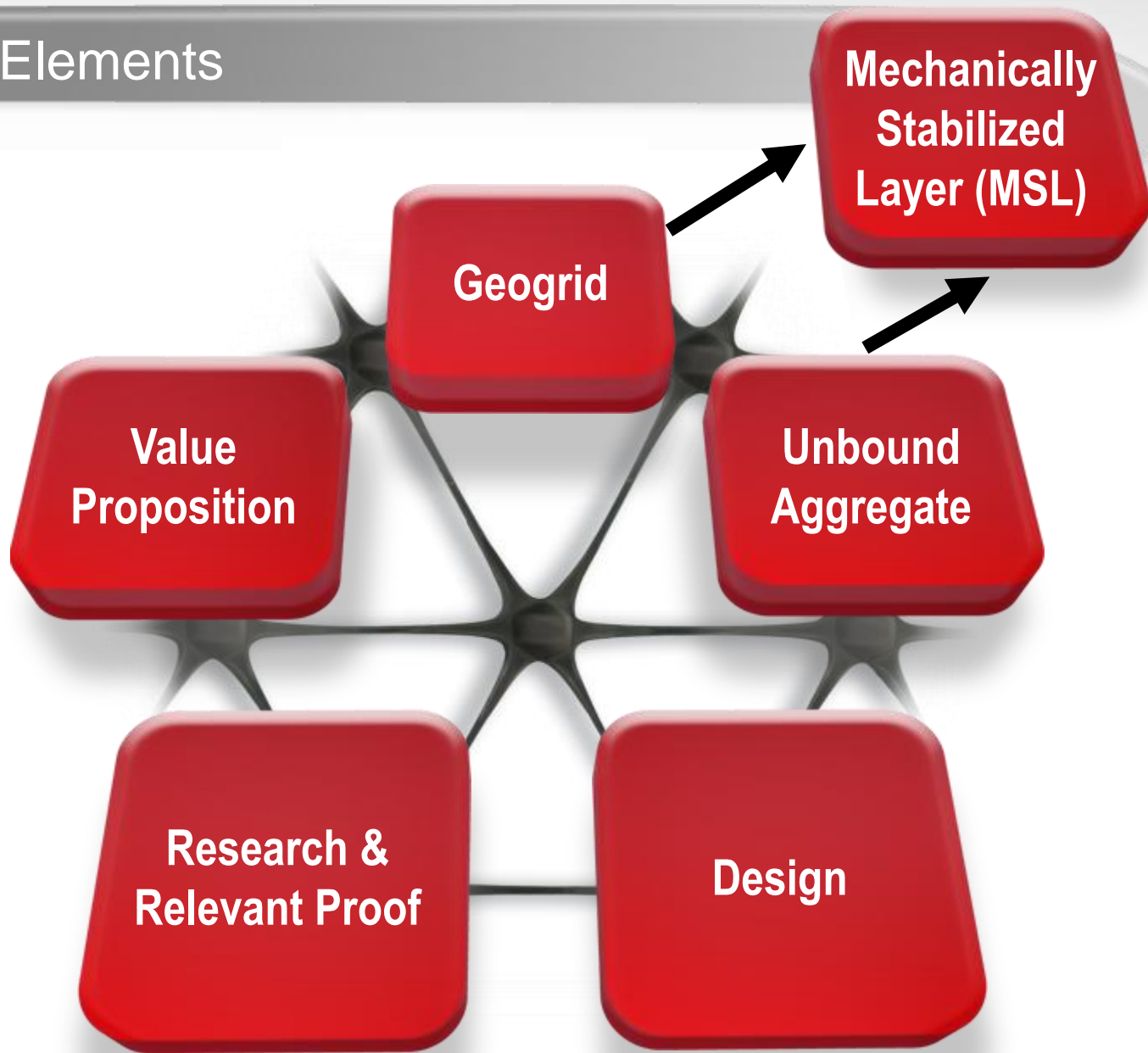
PAVEMENT BASE REINFORCEMENT



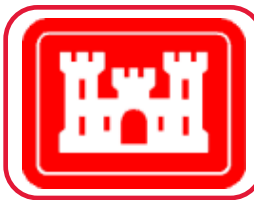
↑ LIFE : ↓ COST



Design Elements



Geogrid Research



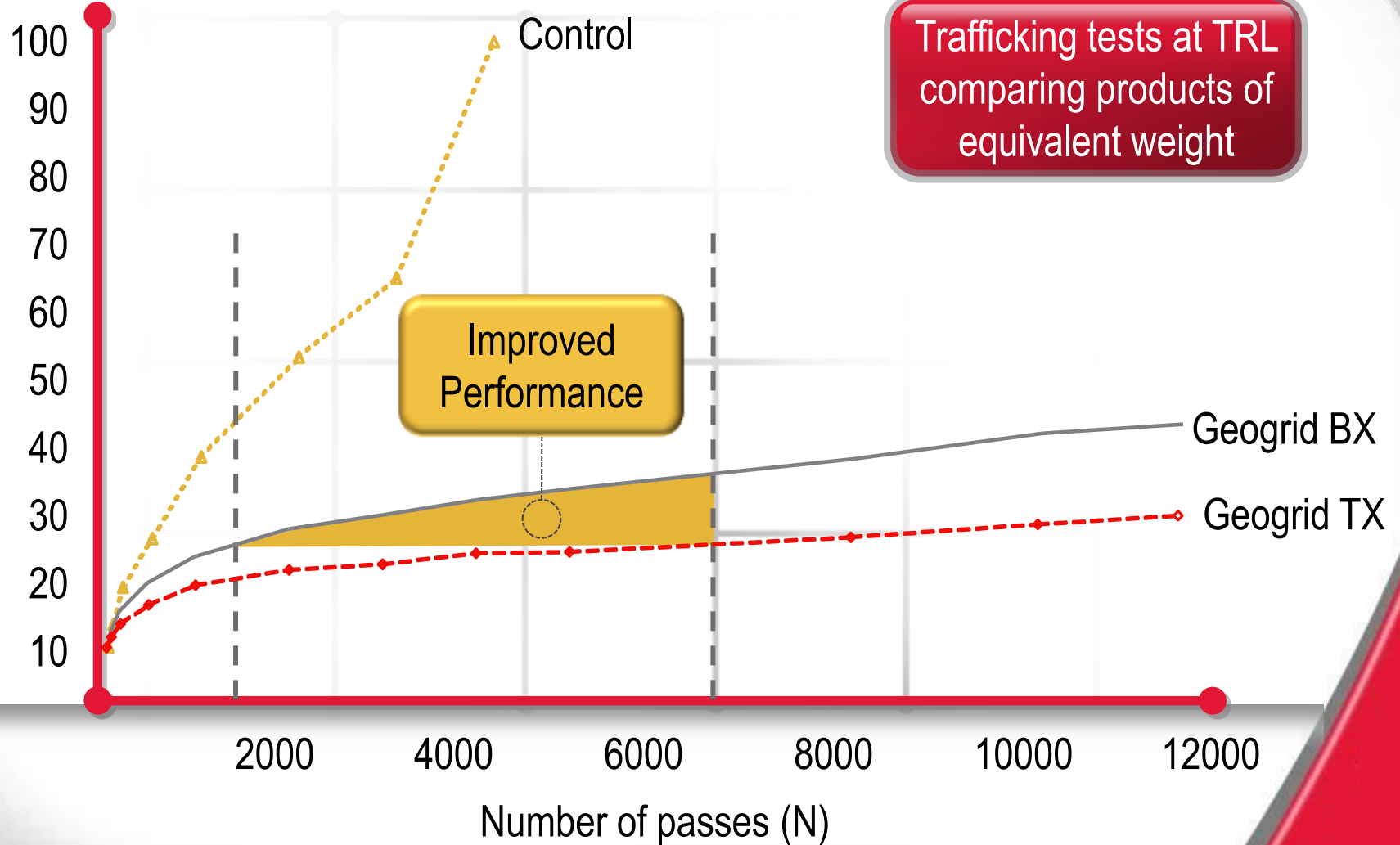
Full-scale Trafficking – TRL



Longer Design Life

Deformation in Wheel path

Deformation (mm)



Trafficking tests at TRL
comparing products of
equivalent weight

Improved
Performance

Geogrid BX

Geogrid TX

Small-scale Trafficking



Performance Measured through Rutting Profiles



Aggregate Rutting Profiles



Unreinforced

3,000 axle passes



Geogrid BX

10,000 axle passes



Geogrid TX

10,000 axle passes

Subgrade Rutting Profiles - Unreinforced



Unreinforced

3,000 axle passes



Geogrid BX

10,000 axle passes



Geogrid TX

10,000 axle passes

Tri-Axial Cell Testing

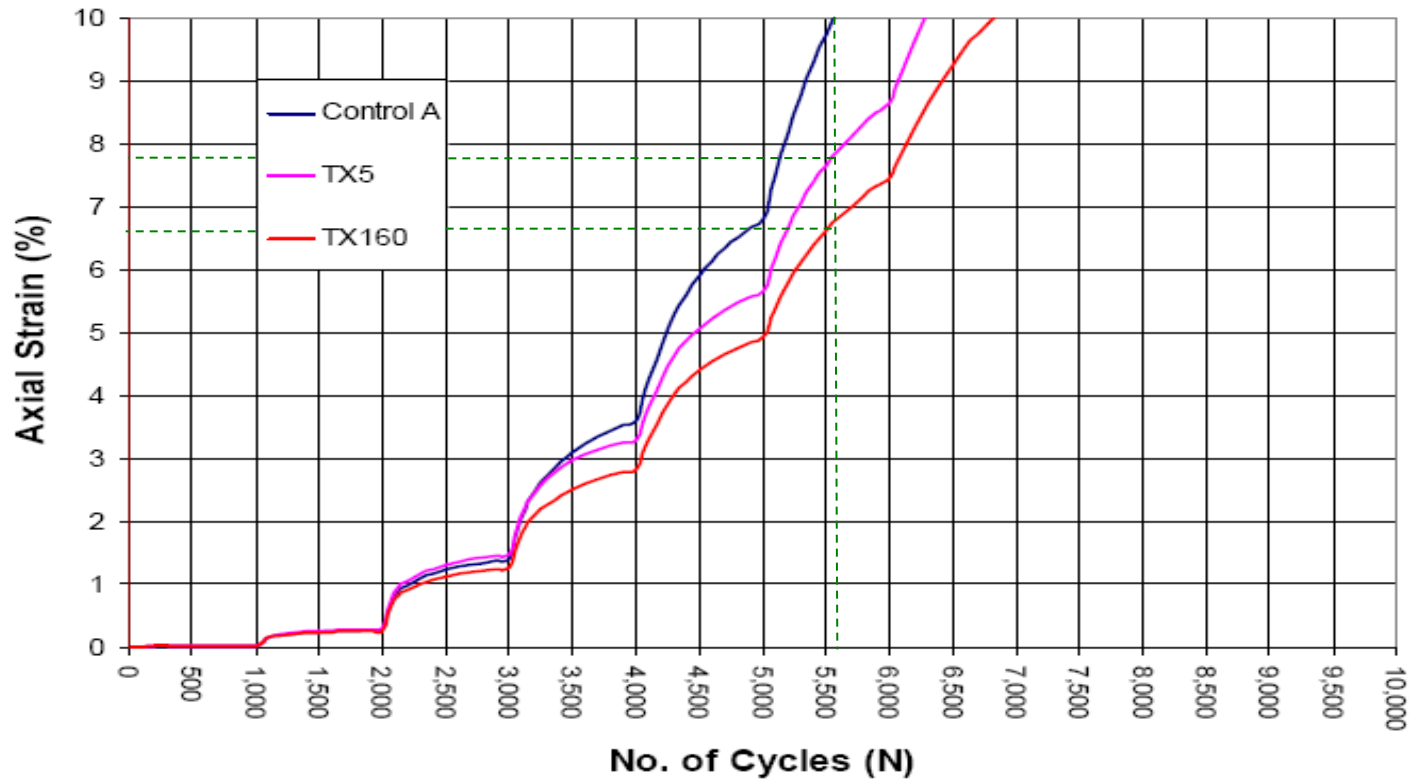


Geogrid Placement



Multistage Repeated Load Tri-Axial Cell Testing

PERCENT STRAIN VS. NUMBER OF CYCLES



Multistage Repeated Load Tri-Axial Cell Testing

$$E = s/v$$

$$E_c = s/0.10 = 10s$$

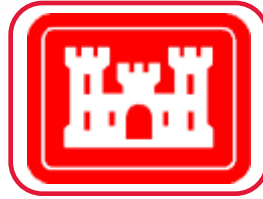
$$E_{TX5} = s/0.0775 = 13s$$

$$E_{TX160} = s/0.066 = 15s$$

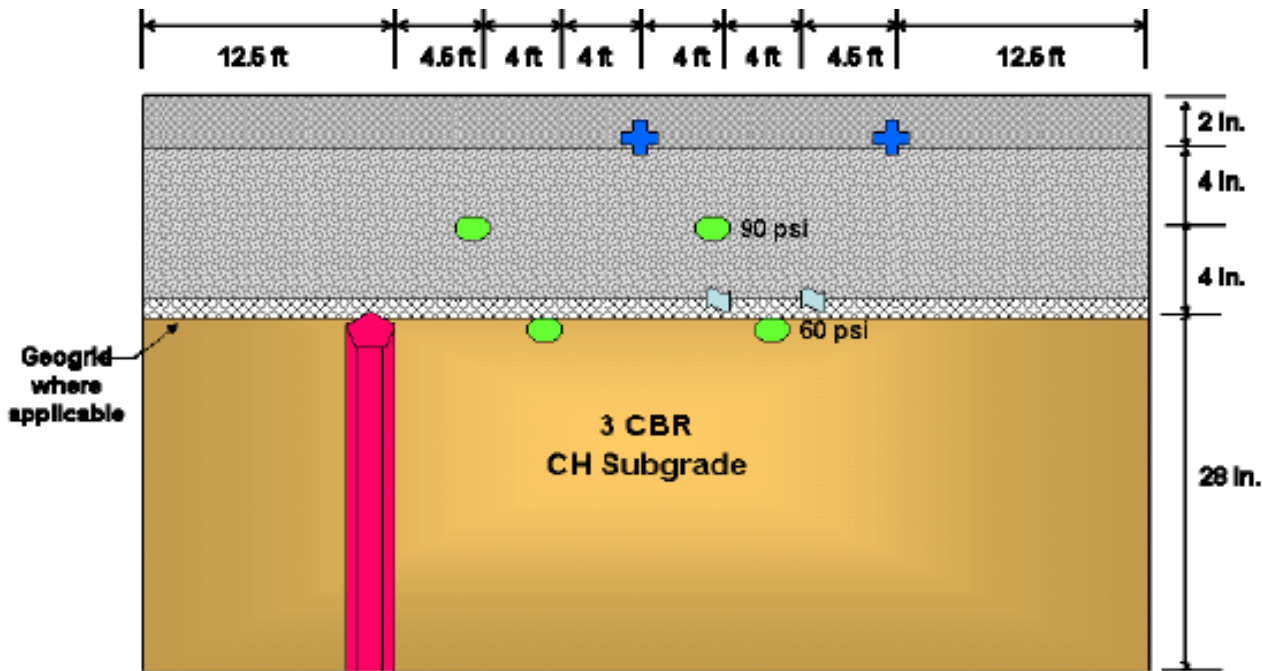
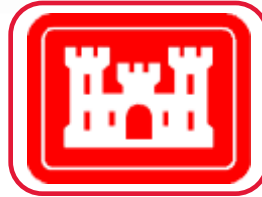
Geogrid 1 = 30% modulus increase

Geogrid 2 = 50% modulus increase

U.S. Army Corps of Engineers Full Scale APT



U.S. Army Corps of Engineers Full Scale APT



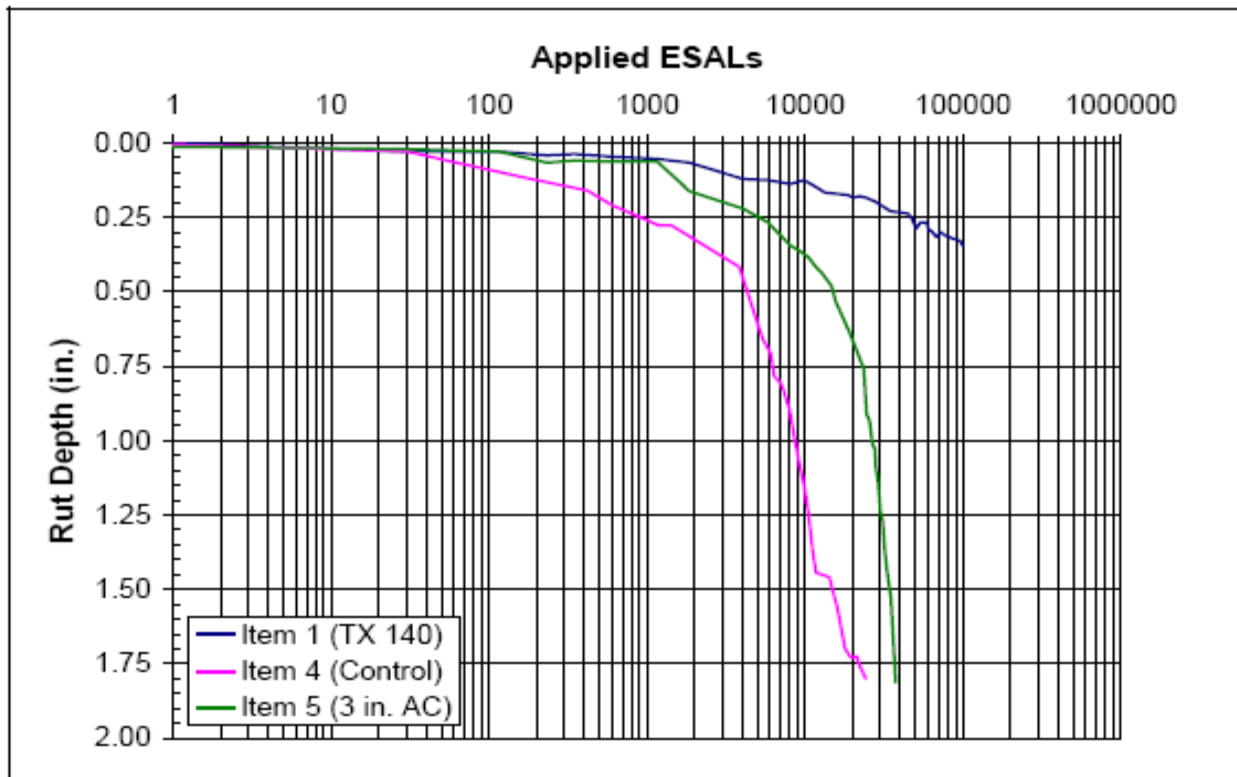
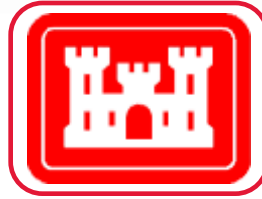


Figure 23. Accumulation of rutting at selected locations

Subgrade Stabilization

Design

Circa 1910









Geosynthetic Design Methods for Subgrade Stabilization

**FHWA Geosynthetic Design and
Construction Guidelines – 2008
Sections 5.4 and 5.5**

FHWA Geosynthetic Design and Construction Guidelines

- Section 5.4 – Design Guidelines for the use of Geotextiles in Temporary and Unpaved Roads
 - 1977 Steward, et. al. (US Forestry Service)
- Section 5.5 – Design Guidelines for the use of Geogrids in Temporary and Unpaved Roads
 - 2003 Tingle and Webster (U.S.A.C.O.E.)
 - 2004 Giroud and Han
- A temporary or unpaved road may constitute improved subgrade or a construction platform for a permanent road

1977 - Steward, et. al. (U.S. Forestry Service)

- Geotextile Only
- Utilizes Bearing Capacity Factor - N_c
- Developed prior to the existence of biaxial geogrid

1977 - Steward, et. al. (U.S. Forestry Service)

“the following limitations apply:

- **the aggregate layer must be**
 - a) compacted to CBR 80,**
 - b) cohesionless (nonplastic);**
- **vehicle passes less than 10,000;**
- **geotextile survivability criteria must be considered; and**
- **subgrade undrained shear strength less than about 90 kPa (CBR <3)”**

Source: U.S.F.S. Steward, et. al. 1977

2003 - Tingle and Webster (U.S. Army Corps of Engineers)

- Modified Steward, et. al. for geogrids and geotextiles
- Utilizes Bearing Capacity Factor - N_c
- Identifies 3 performance mechanisms of geogrid

Geogrid Mechanisms

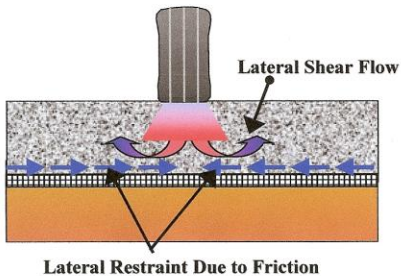


Figure 1. Lateral restraint reinforcement mechanism.

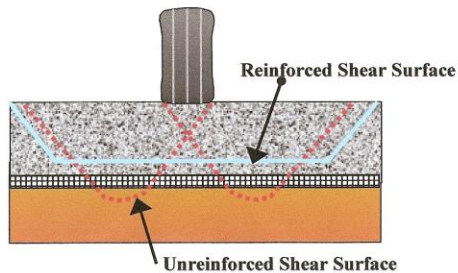


Figure 2. Improved bearing capacity reinforcement mechanism.

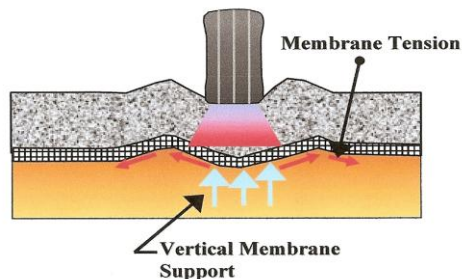


Figure 3. Tensioned membrane effect reinforcement mechanism.

1. Lateral Restraint
"Confinement"
(true stability)

2. Improved Bearing Capacity
"Snowshoe Effect"
(load distribution)

3. Tensioned Membrane
"Hammock Effect"
(initial tension)

Geogrid Mechanisms

“In the early stages of research regarding geogrid reinforcement of pavement systems, the tensioned membrane effect was thought to be the primary reinforcement mechanism. However, subsequent investigations have shown that reinforcement benefits are obtained without significant deformation of the pavement section. Thus, lateral restraint has been identified as the primary reinforcement mechanism, followed by the improved bearing capacity concept and the tensioned membrane effect.

2004: Giroud and Han

- Geogrid and geotextile
- Built on principles of Giroud and Noiray (1981) design method for geotextiles
- Individual geogrids must be empirically calibrated to formula
- Utilizes Bearing Capacity Factor – N_c
- Utilizes Bearing Capacity Mobilization Coefficient – m
- Utilizes Stress Distribution Angle – α

2004: Giroud and Han

“Due to their large apertures, geogrids may interlock with base course aggregate if there is an appropriate relationship between geogrid aperture size and aggregate particle size. While the degree of interlocking depends on the relationship between geogrid aperture size and aggregate particle size, the effectiveness of interlocking depends on the in-plane stiffness of the geogrid and the stability of the geogrid ribs and junctions. As a result of interlocking, the mechanisms of unpaved structure reinforcement are different for geotextiles and geogrids.”

FHWA Design Guidelines Summary

- Only three (3) design methods detailed for soft subgrades
- Steward, et. al. only pertains to geotextiles
- Tingle and Webster built on Steward's study and developed geogrid performance mechanisms
- Giroud and Han introduce bearing capacity mobilization and stress distribution factors for geogrid

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Traffic Conditions

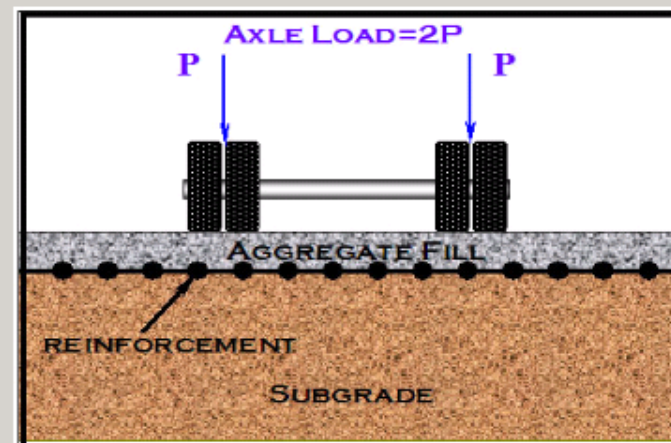
Axle load (kips)
 Tire pressure (psi)
 Axle passes (each)
 Maximum rut depth (in)

Soil Conditions

Aggregate Fill CBR (%)
 Design Subgrade CBR (%)

Geogrid Reinforcement

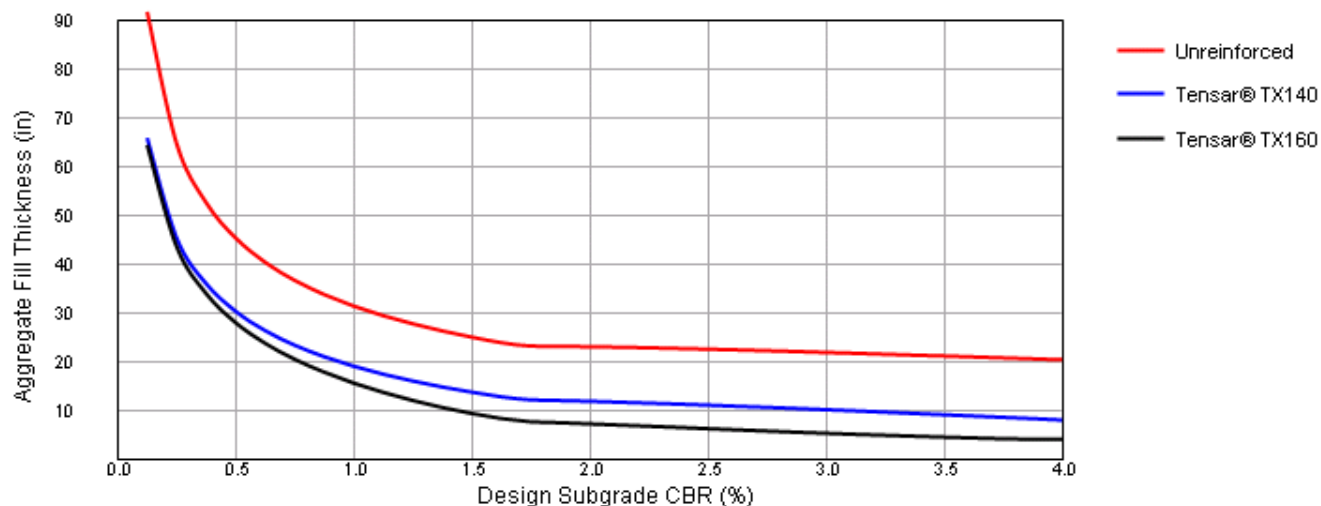
Unreinforced
 TX5 TX7
 TX140 TX160



Geosynthetic	Aggregate Fill Thickness (in)		Aggregate Fill Thickness Savings	
	Calculated	Required	(in)	(%)
Unreinforced	24.05	25	N/A	N/A
Tensar® TX140	12.9	13	12	48
Tensar® TX160	8.46	9	16	64

Click Here to Conduct
Unpaved Application
Cost Analysis

Click Here to Check
Subgrade Separation



Unpaved Application Cost Analysis - Data Input

Results

Project Size (ft)

Length Width

Aggregate Cost

Total In-Place Cost (\$/Ton) (\$/CY)

Aggregate Fill Thickness (in)

Unreinforced TX5 TX7 TX140 TX160

Top Surface Constraint

 Undercut required (Fixed top grade) Finished grade is (in) existing gradeUndercut and removal cost (\$/CY) (\$/Ton) (\$/CY)Cost of placed and compacted aggregate to (\$/Ton)build up to finished level, if required (\$/CY) (\$/CY) No undercut (Free top grade)

Geogrid Cost

[Required minimum overlap \(ft\)](#) Installation cost (\$/SY) [Delivered to site cost \(\\$/SY\)](#)

	TX5	TX7	TX140	TX160
Delivered to site cost (\$/SY)	<input type="text" value="3.00"/>	<input type="text" value="5.00"/>	<input type="text" value="2.50"/>	<input type="text" value="4.00"/>
Roll width (ft)	<input type="text" value="13.1"/>	<input type="text" value="13.1"/>	<input type="text" value="13.1"/>	<input type="text" value="13.1"/>

Legend

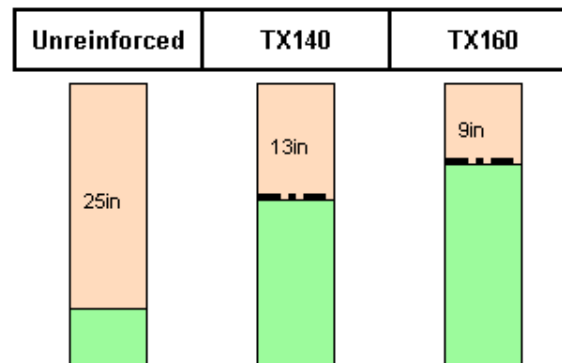
Aggregate fill

Existing subgrade

Additional fill required

Geogrid

[Click Here to Modify Design Parameters](#)

 Proposed Grade
Existing Grade


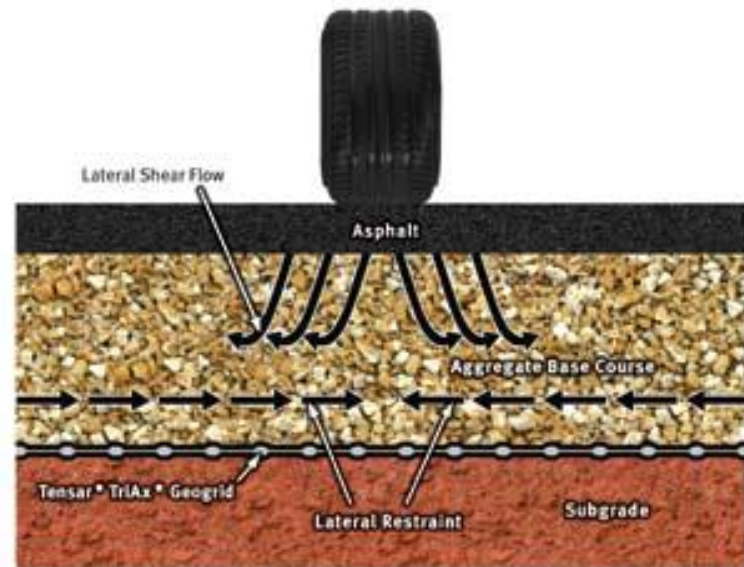
Aggregate costs	\$129,871	\$67,533	\$46,753
Geogrid costs	\$0	\$24,014	\$36,021
Undercut costs	\$24,113	\$12,539	\$8,681
Additional fill costs	\$0	\$0	\$0
Total project costs	\$153,983	\$89,610	\$105,619
Overall project savings	\$0	\$49,898	\$62,528
Percent savings	0%	32%	41%



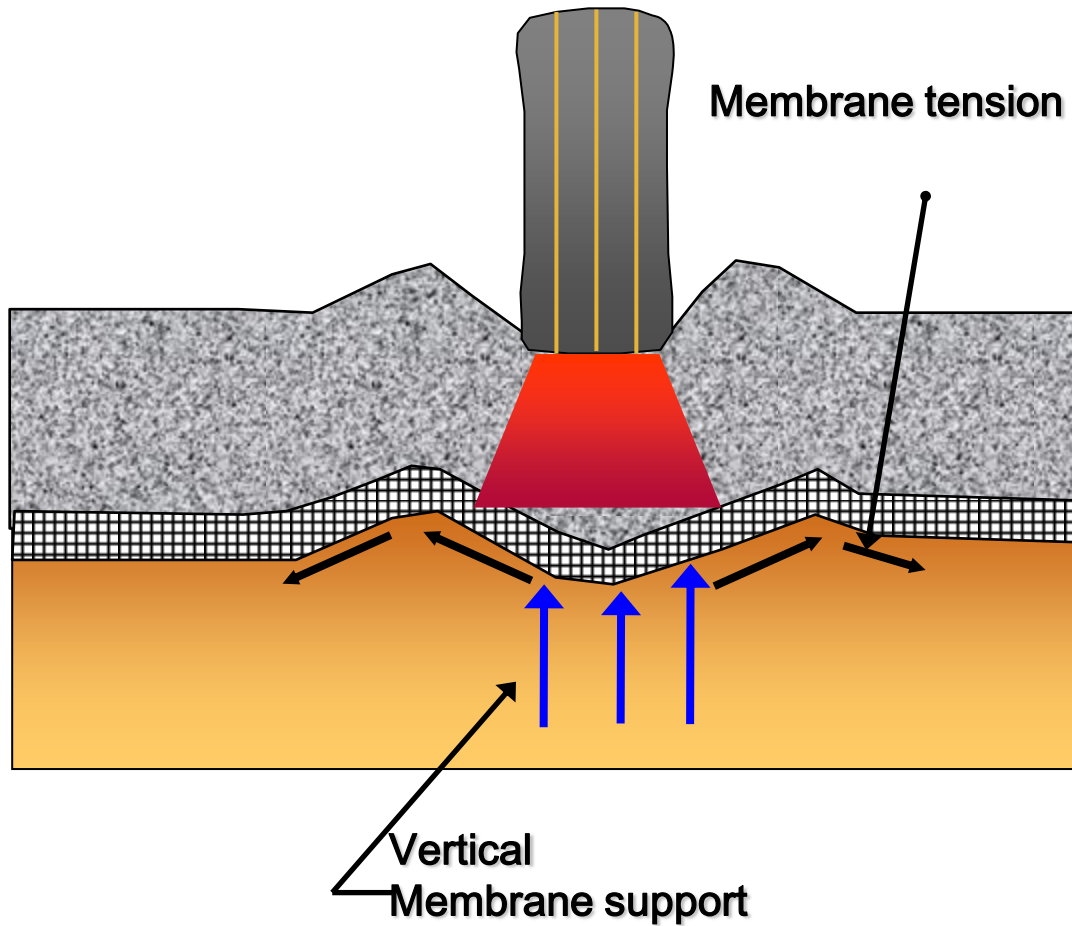
Pavement Design with Geogrid

Pavement Base Reinforcement

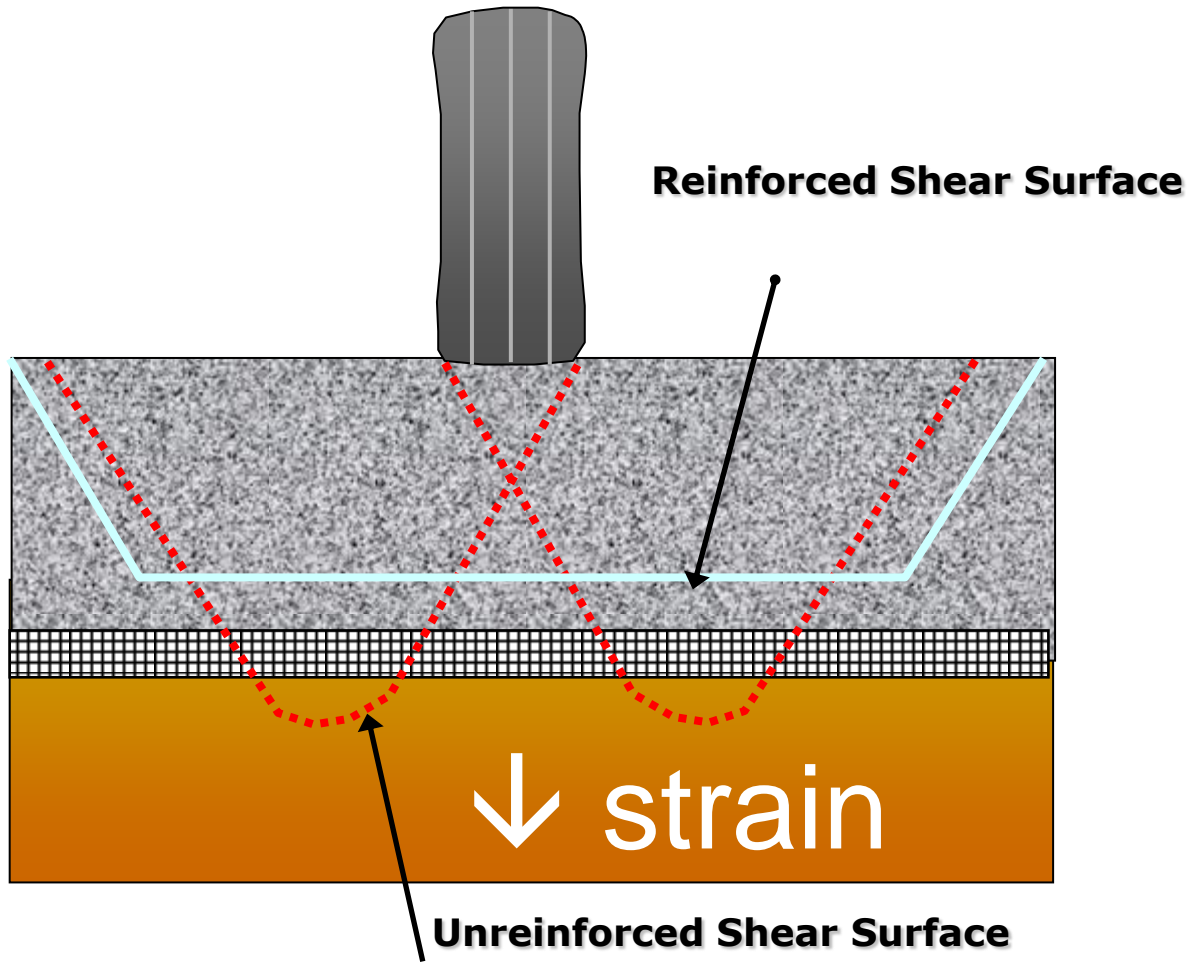
- Equal performance for considerably less cost
- Increased performance with lower life cycle costs
- Optimized – Increased performance for the same cost



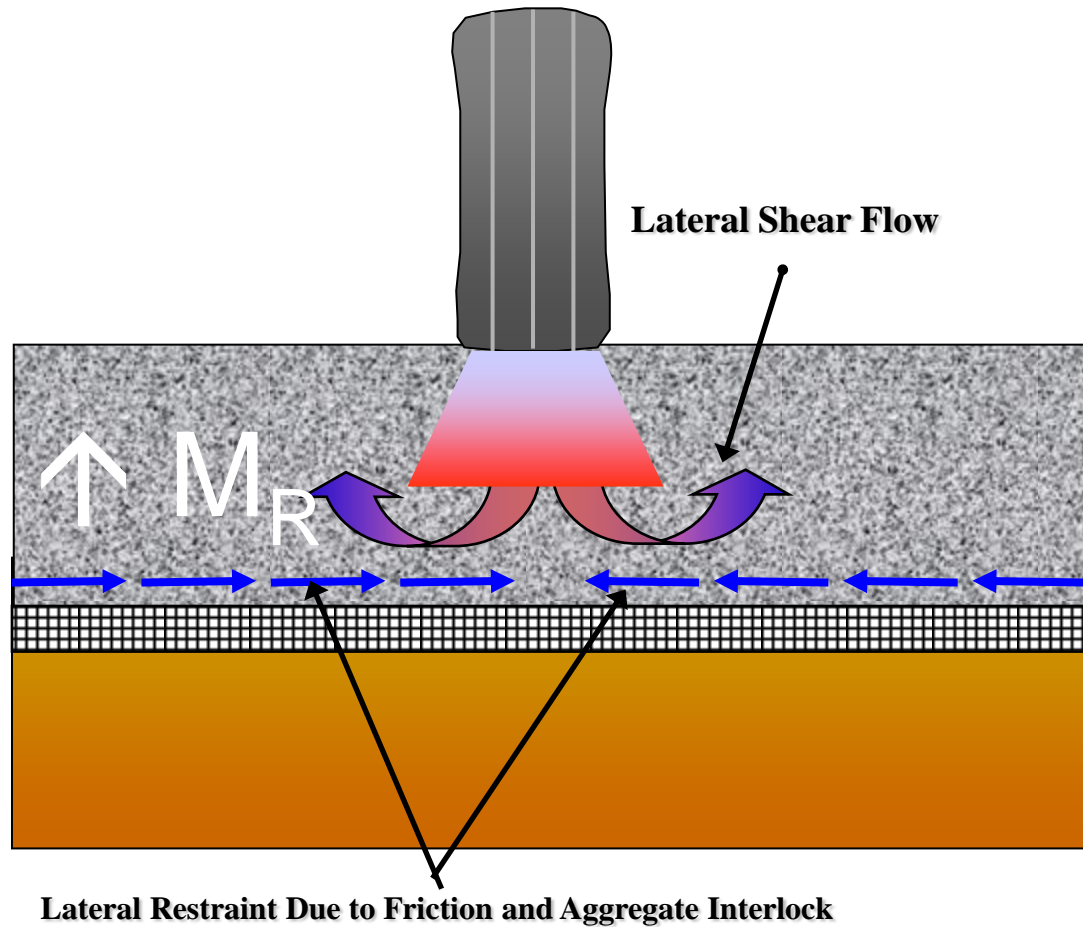
Mechanisms – Tensile Membrane Effect



Mechanisms – Improved Bearing Capacity



Mechanisms – Lateral Restraint



AASHTO 1993 Pavement Design Formula

$$\log_{10}(W_{18}) = Z_R \times S_o + 9.36 \times \log_{10}(SN+1) - 0.20 + \frac{\log_{10}\left(\frac{\Delta PSI}{4.2-1.5}\right)}{0.40 + \frac{1094}{(SN+1)^{5.19}}} + 2.32 \times \log_{10}(M_R) - 8.07$$

W_{18} = predicted number of 80 kN (18,000 lb.) ESALs

Z_R = standard normal deviate

S_o = combined standard error of the traffic prediction and performance prediction

S_N = Structural Number (an index that is indicative of the total pavement thickness required)

$$= a_1 D_1 + a_2 D_2 m_2 + a_3 D_3 m_3 + \dots$$

a_i = i^{th} layer coefficient

D_i = i^{th} layer thickness (inches)

m_i = i^{th} layer drainage coefficient

DPSI = difference between the initial design serviceability index, p_o , and the design terminal serviceability index, p_t

M_R = subgrade resilient modulus (in psi)

Standard Practice for

Geosynthetic Reinforcement of the Aggregate Base Course of Flexible Pavement Structures

AASHTO Designation: R 50-09¹



1. SCOPE

- 1.1. This standard practice provides guidance to pavement designers interested in incorporating geosynthetics for the purpose of reinforcing the aggregate base course of flexible pavement structures. Geosynthetic reinforcement is intended to provide structural support of traffic loads over the life of the pavement.
 - 1.1.1. For the purpose of this guide, base reinforcement is the use of a geosynthetic within, or directly beneath, the granular base course.
 - 1.1.2. When referring to geosynthetics, the discussion is limited to geotextiles, geogrids, or geogrid/geotextile composites.

2. REFERENCED DOCUMENTS

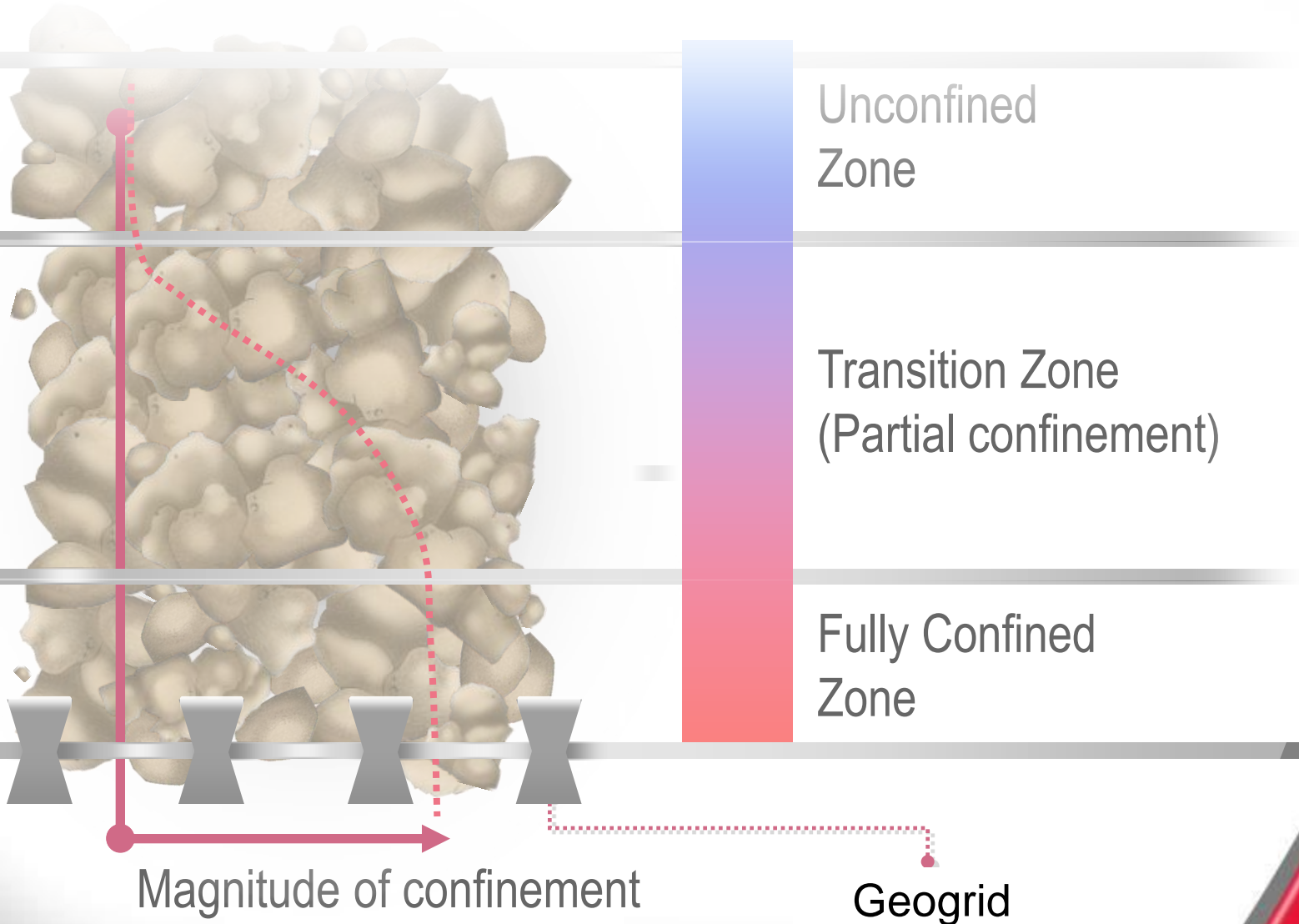
6.1 Geosynthetics are used in the pavement structure for structural support of traffic loads over the design life of the pavement. The geosynthetic is expected to provide one or both of these benefits: 1) improved or extended service life of the pavement, or 2) a reduction in the thickness of the structural section.

AASHTO Designation: R 50-09¹



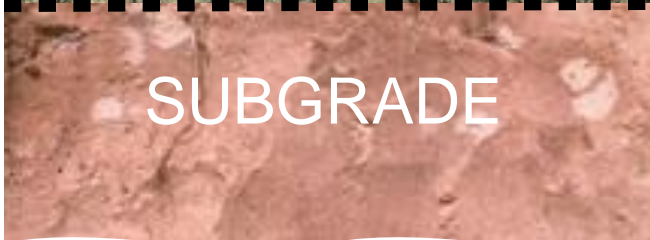


7.1.6.1 The traffic benefit ratio (TBR) is defined as the ratio of the number of load cycles of a reinforced pavement to reach a defined failure state, to the number of loads for the same unreinforced section to reach the same defined failure state.

Mechanically Stabilized Layer (MSL)



Typical Pavement Section

 <p>HMA</p>	<p>ACC: $a_i = 0.36$ to 0.44</p>
 <p>AGGREGATE BASE</p>	<p>ABC: $a_i = 0.12$ to 0.18 ↑ stress = ↑ M_r MSL: a_i up to 0.28</p>
 <p>SUBGRADE</p>	<p>↓ stress = ↓ strain</p>

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Paved Application Design Analysis - Data Input Results

Material Properties

Layer Name	Layer Coefficient	Drainage Factor
ACC1 Asphalt Wearing Course	0.44	
ACC2 Dense-graded Asphalt Course	0.36	
ABC Aggregate Base Course	0.14	1.0
SBC Subbase Course	0.08	1.0

Design Traffic (ESALs)

100,000

Unreinforced Section

Layer Name	Thickness (in)
<input checked="" type="checkbox"/> ACC1	2.00
<input checked="" type="checkbox"/> ACC2	4.00
<input checked="" type="checkbox"/> ABC	12.00
<input type="checkbox"/> SBC	6.00

Tensor TriAx™ Geogrid Reinforced Section

Layer Name	Thickness (in)	Tensor Geogrid Reinforcement
<input checked="" type="checkbox"/> ACC1	2.00	
<input checked="" type="checkbox"/> ACC2	3.00	
<input checked="" type="checkbox"/> MSL	8.75	TX5
<input type="checkbox"/> SBC	6.00	

Reliability (%)

90

Standard Normal Deviate

-1.282

Standard Deviation

0.49

Subgrade Resilient Modulus (psi)

5000

Serviceability Initial

4.5

Terminal

2.5



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Paved Application Design Analysis - Data Input Results

Design Traffic (ESALs) = 1,000,000

Round Results

[Click Here to Conduct Paved Application Cost Analysis](#)

Unreinforced Section

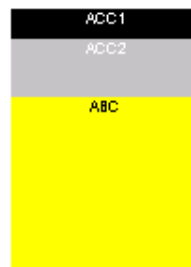
Layer	Di	ai	mi	SN
ACC1	2.00	0.440	N/A	0.88
ACC2	4.00	0.360	N/A	1.44
ABC	12.00	0.140	1.0	1.68
Overall Structural Number (SN)				4.00

Thickness (in)

ACC1:

ACC2:

ABC:



Tensor TriAx™ Geogrid Reinforced Section

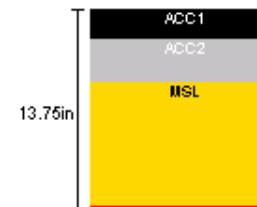
Layer	Di	ai	mi	SN
ACC1	2.00	0.440	N/A	0.88
ACC2	3.00	0.360	N/A	1.08
MSL	8.75	0.234	1.0	2.05
Overall Structural Number (SN)				4.01

Thickness (in)

ACC1:

ACC2:

MSL:



Geogrid Depth

TX5

1,066,000

Calculated Traffic
(ESAL's)

1,084,000

Paved Application Cost Analysis - Data Input

Results

Project Size (ft)

Length Width

Top Surface Constraint

 Milling or Undercut required (Fixed top grade)
 No undercut or Milling (Free top grade)
Finished grade (in) existing gradeMilling or undercut and removal cost (\$/CY) Cost of placed and compacted aggregate to build up to finished level, if required (\$/CY)

Material Cost

	Layer Name	Installed Cost (\$/CY)	
ACC1:	Asphalt Wearing Course	140	
ACC2:	Dense-graded Asphalt Course	129.87	
ABC:	Aggregate Base Course	35.91	
SBC:	Subbase Course	30	

Tensor TriAx Geogrid Cost

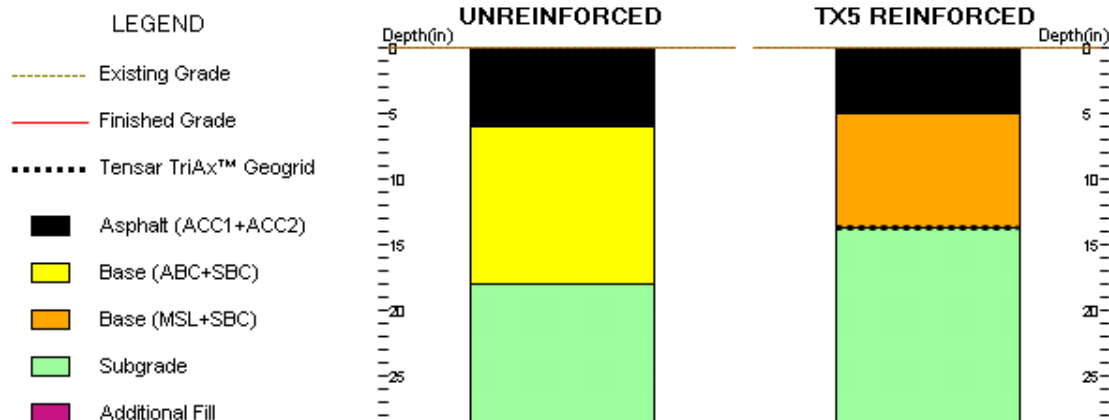
Geogrid Type	Roll Width (ft)	Installed Cost (\$/SY)
TX5	<input type="text" value="13.1"/>	<input type="text" value="3.50"/>

Unreinforced Section

Layer Name	Thickness (in)
ACC1	2.00
ACC2	4.00
ABC	12.00

Tensor TriAx™ Geogrid Reinforced Section

Layer Name	Thickness (in)	Tensor Geogrid Reinforcement
ACC1	2.00	
ACC2	3.00	
MSL	8.75	TX5



[Allowable Traffic \(ESALs\):](#)

Section Unit Cost:

% Life Extension:

% Initial Cost Saving:

1,066,000

\$36.68/SY

1,084,000

\$33.01/SY

2%

10%

INITIAL CONSTRUCTION COSTS

Round Results

Asphalt (ACC1+ACC2)	\$ 325,714	\$ 272,804
Aggregate Base (ABC/MSL)	\$ 175,560	\$ 128,013
Subbase (SBC)	\$ 0	\$ 0
Milling or Undercut	\$ 36,667	\$ 28,009
Additional Fill Costs	\$ 0	\$ 0
Geogrids	\$ 0	\$ 55,252
Overall Project Cost	\$ 537,941	\$ 484,078

Click Here to Perform
a Life Cycle Cost
Analysis

Click Here to Modify
Pavement Structure

Design Traffic (ESALs) = 1,000,000

Round Results

[Click Here to Conduct Paved Application Cost Analysis](#)

Unreinforced Section

Layer	Di	ai	mi	SN
ACC1	2.00	0.440	N/A	0.88
ACC2	4.00	0.360	N/A	1.44
ABC	12.00	0.140	1.0	1.68
Overall Structural Number (SN)				4.00

Tensar TriAx™ Gegrid Reinforced Section

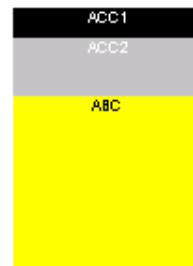
Layer	Di	ai	mi	SN
ACC1	2.00	0.440	N/A	0.88
ACC2	3.00	0.360	N/A	1.08
MSL	12.00	0.215	1.0	2.58
Overall Structural Number (SN)				4.54

Thickness (in)

ACC1:

ACC2:

ABC:

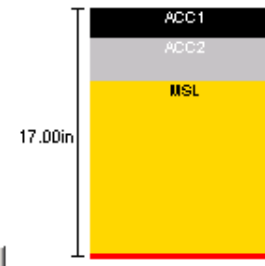


Thickness (in)

ACC1:

ACC2:

MSL:



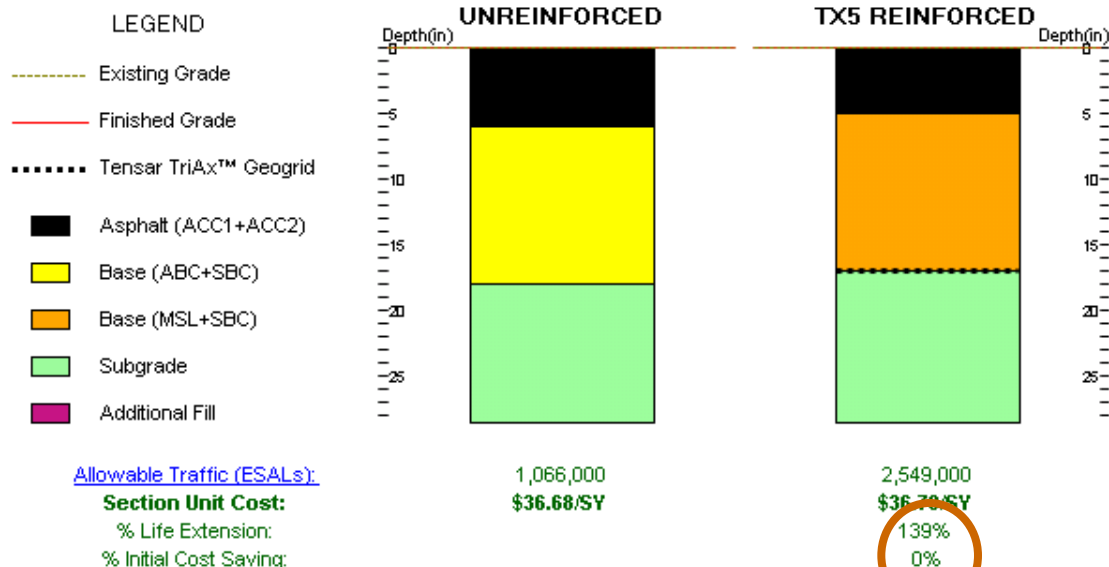
Geogrid Depth

TX5

1,066,000

Calculated Traffic
(ESAL's)

2,549,000

**INITIAL CONSTRUCTION COSTS** Round Results

Asphalt (ACC1+ACC2)	\$ 325,714	\$ 272,804
Aggregate Base (ABC/MSL)	\$ 175,560	\$ 175,560
Subbase (SBC)	\$ 0	\$ 0
Milling or Undercut	\$ 36,667	\$ 34,630
Additional Fill Costs	\$ 0	\$ 0
Geogrids	\$ 0	\$ 55,252
Overall Project Cost	\$ 537,941	\$ 538,246

Click Here to Perform
a Life Cycle Cost
Analysis

Click Here to Modify
Pavement Structure

Material Properties

	Layer Name	Layer Coefficient	Drainage Factor
ACC1	Asphalt Wearing Course	0.44	
ACC2	Dense-graded Asphalt Course	0.36	
ABC	Aggregate Base Course	0.14	1.0
SBC	Subbase Course	0.08	1.0

Design Traffic (ESALs)

100,000

Unreinforced Section

Layer Name	Thickness (in)
<input checked="" type="checkbox"/> ACC1	2.00
<input checked="" type="checkbox"/> ACC2	4.00
<input checked="" type="checkbox"/> ABC	12.00
<input type="checkbox"/> SBC	6.00

Tensor TriAx™ Geogrid Reinforced Section

Layer Name	Thickness (in)	Tensor Geogrid Reinforcement
<input checked="" type="checkbox"/> ACC1	2.00	
<input checked="" type="checkbox"/> ACC2	3.00	
<input checked="" type="checkbox"/> MSL	12.00	TX5
<input type="checkbox"/> SBC	6.00	

Reliability (%)

90

Standard Normal Deviate

-1.282

Standard Deviation

0.49

Subgrade Resilient Modulus (psi)

5000

Serviceability Initial

4.5

Terminal

3.25

Design Traffic (ESALs) = 1,000,000

Round Results

[Click Here to Conduct Paved Application Cost Analysis](#)

Unreinforced Section

Layer	Di	ai	mi	SN
ACC1	2.00	0.440	N/A	0.88
ACC2	4.00	0.360	N/A	1.44
ABC	12.00	0.140	1.0	1.68
Overall Structural Number (SN)				4.00

Tensor TriAx™ Geogrid Reinforced Section

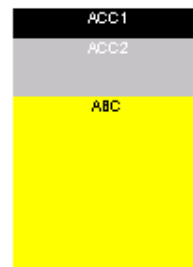
Layer	Di	ai	mi	SN
ACC1	2.00	0.440	N/A	0.88
ACC2	3.00	0.360	N/A	1.08
MSL	12.00	0.215	1.0	2.58
Overall Structural Number (SN)				4.54

Thickness (in)

ACC1:

ACC2:

ABC:

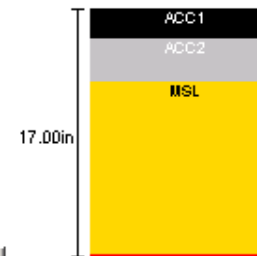


Thickness (in)

ACC1:

ACC2:

MSL:



Geogrid Depth

TX5

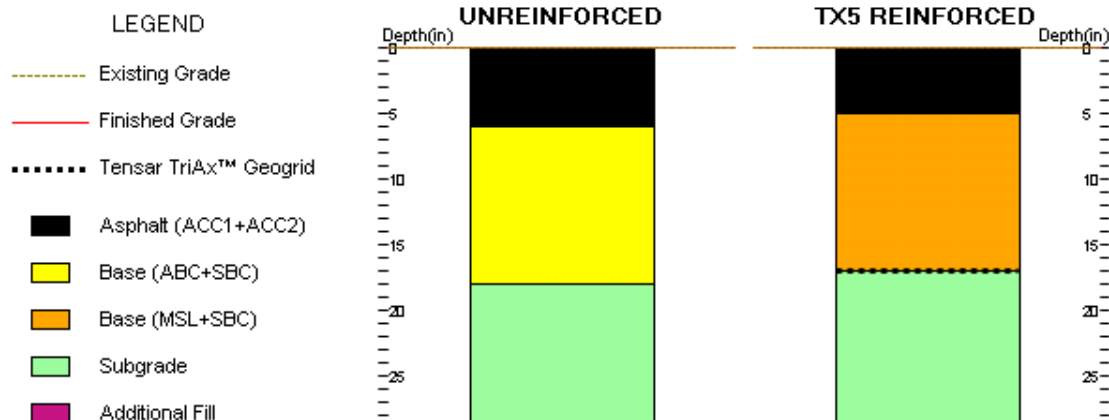
522,000

Calculated Traffic
(ESAL's)

1,087,000

Paved Application Cost Analysis - Data Input

Results



Allowable Traffic (ESALs):

Section Unit Cost:

% Life Extension:

% Initial Cost Saving:

522,000

\$36.68/SY

1,087,000

\$36.70/SY

100%

0%

INITIAL CONSTRUCTION COSTS

Round Results

Asphalt (ACC1+ACC2)	\$ 325,714	\$ 272,804
Aggregate Base (ABC/MSL)	\$ 175,560	\$ 175,560
Subbase (SBC)	\$ 0	\$ 0
Milling or Undercut	\$ 36,667	\$ 34,630
Additional Fill Costs	\$ 0	\$ 0
Geogrids	\$ 0	\$ 55,252
Overall Project Cost	\$ 537,941	\$ 538,246

Click Here to Perform
a Life Cycle Cost
Analysis

Click Here to Modify
Pavement Structure

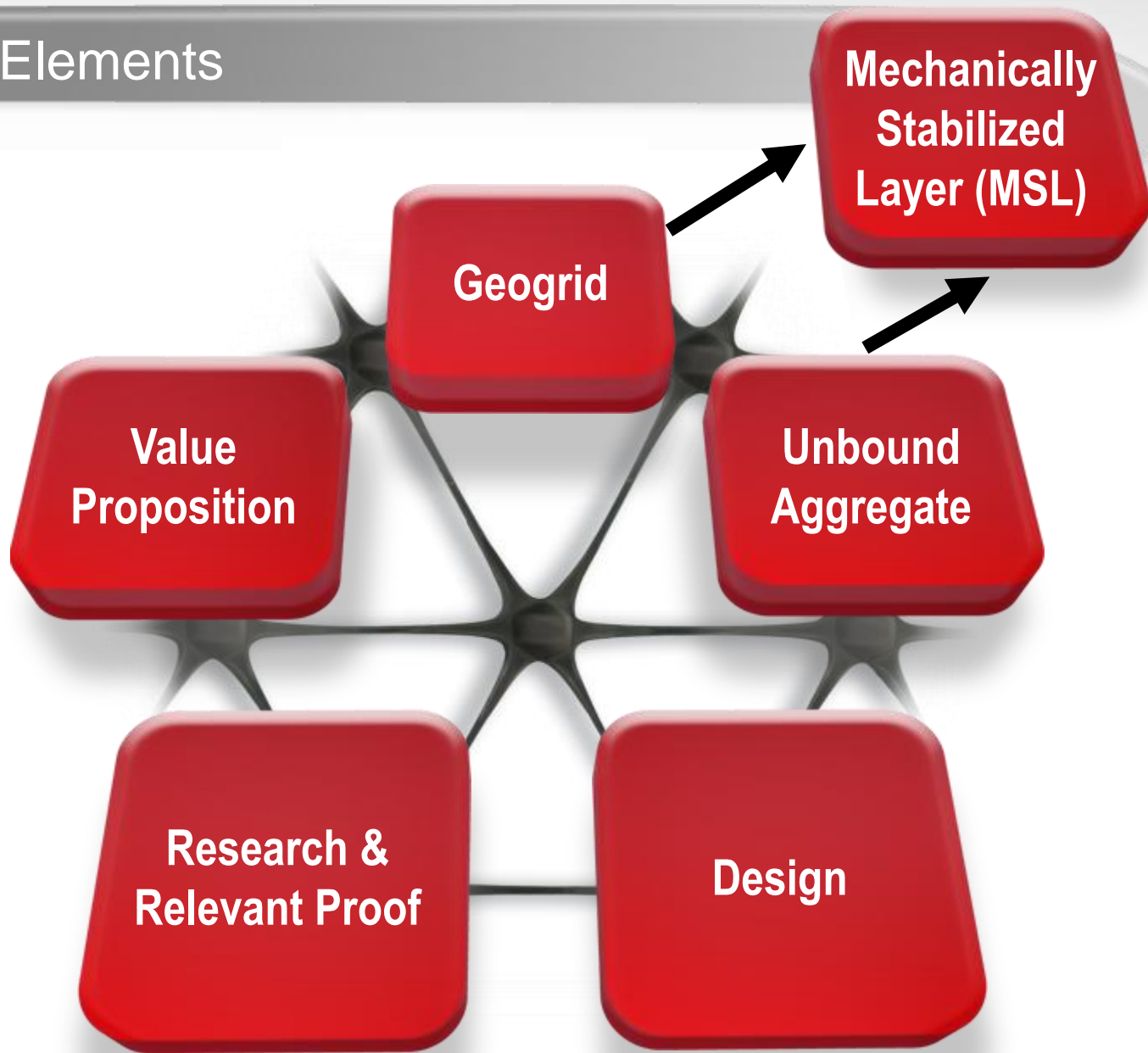
Pavement Design Summary

Pavement	HMA Thickness	Agg Base Thickness	ESALs	+ / -	Svcblty Loss	Cost \$/SY	+ / -
Original Design	6"	12"	1,066,000	N.A.	2	\$36.68	N.A.
Reinforced for Maximum Cost Savings	5"	8.75"	1,084,000	2%	2	\$33.01	-10%
Reinforced for Maximum Performance Cost Neutral	5"	12"	2,549,000	139%	2	\$36.70	0%
Reinforced for Optimized Serviceability Cost Neutral	5"	12"	1,087,000	2%	1.25	\$36.70	0%



Specifications

Design Elements



2003 - Tingle and Webster (U.S. Army Corps of Engineers)

“Extruded geogrids are formed using a polymer sheet that is punched and drawn in either one or two directions for improvement of engineering properties. Extruded geogrids have shown good performance when compared to other types for pavement reinforcement applications.”

Source: U.S. Army Corps of Engineers ETL 1110-1-189 (Tingle and Webster)

2004: Giroud and Han

“In addition to the aperture stability modulus, other properties of geogrids likely affect their ability to interlock with aggregate and efficiently reinforce the base course. Tensile modulus at small strains is one obvious property that is important. Webster (1992) listed rib thickness, rib cross-section shape, junction strength, and aperture size as properties, in addition to aperture stability, that affect performance of geogrid-reinforced bases.”

Source: ASCE August 2004 *Journal of Geotechnical and Geoenvironmental Engineering*

AASHTO Designation: R 50-09

3.1. Because the benefits of geosynthetic reinforced pavement structures may not be derived theoretically, test sections are necessary to obtain benefit quantification. Studies have been done that demonstrate the value added by a geosynthetic in a pavement structure. These studies, necessarily limited in scope, remain the basis for design in the field.

AASHTO Designation: R 50-09

5.2 Design procedures use experimentally derived input parameters that are often geosynthetic specific. Thus, computed engineering designs and economic benefits are not easily translated to other geosynthetics. Therefore, users of this document are encouraged to affirm their designs with field verification of the reinforced pavement performance, both in engineering design and economic benefits.

Geogrid Specifications

- **Material Specification**
 - Specify only geogrid properties
 - Difficult to assess performance based on geogrid index properties only
 - Basis of design can be compromised
- **System Specification**
 - Specify performance of composite geogrid / aggregate system
 - Ensures an acceptable performance level
 - Protects your design
 - Uses results from full-scale testing to predict performance
 - Design methodologies endorsed by FHWA and AASHTO already exist

Material Specification Excerpt

•Properties	•MD Values ¹	•CMD Values ¹	•General ¹
•Aperture Dimensions ² , mm(in)	•25 (1.0)	•33 (1.3)	
•Minimum Rib Thickness ²	•1.27 (0.05)	•1.27 (0.05)	
•Rib shape			•rectangular
•Aperture shape			•rectangular
•Junction Efficiency ³ , %			•93
•Aperture stability ⁴ , m-N/deg			•0.65
•Flexural Stiffness ⁵ , mg-cm			•750,000
•Tensile Strength @ 2% Strain ⁶ , kN/m (lb/ft)	•6.0 (410)	•9.0 (620)	
•Ultimate Tensile Strength ⁶ , kN/m (lb/ft)	•19.2 (1,310)	•28.8 (1,970)	
•Resistance to chemical degradation ⁷			•100%
•Resistance to ultra-violet light and weathering ⁸			•100%

System Specification Excerpt

1. PERFORMANCE CRITERIA

- A. The design of the pavement shall be in accordance with the 1993 American Association of State Highway and Transportation Officials (AASHTO) Guide for Design of Pavement Structures.
- B. The design of the pavement shall be based on the following parameters:
 - (1) Subgrade Resilient Modulus XX psi
 - (2) Serviceability Loss XX
 - (3) Reliability XX %
 - (4) Standard Deviation 0.49
 - (5) Design traffic = XX Equivalent Single Axle Loads (ESALs)
- C. The Mechanically Stabilized Layer (MSL) within the pavement structure shall have a thickness of XX inches or as shown on the contract plans.
- D. The MSL shall be incorporated into the pavement design by using modified layer coefficients. Modified layer coefficients shall be calibrated and validated with the results of full scale laboratory, field and/or accelerated pavement testing where actual geogrids are tested in-soil and in representative conditions.

Summary

- Mechanically Stabilized Layers consisting of engineered composite sections of geogrid and aggregate are a reliable, value added method to improve soil performance.
- Geogrid reinforcement in flexible pavements can be used to design for maximum initial cost savings or optimized designs that produce increased performance at equal cost
- Proven design methods, endorsed by FHWA and AASHTO, can be employed for subgrade stabilization and pavement design.
- In order to ensure a road performs **as intended**, a **performance specification** is recommended

Thank You

See you in the Hallway!